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GENERAL INFORMATION

ANCHORS & FASTENERS

POWER-BOLT®+

Heavy Duty Sleeve Anchor

PRODUCT DESCRIPTION

The Power-Bolt+ anchor is a torque controlled, heavy duty sleeve style anchor which is designed for consistent performance in cracked and uncracked concrete. Suitable base materials include normal-weight concrete and lightweight concrete. The anchor is manufactured with a zinc plated carbon steel bolt, sleeve, cone and expansion clip. The Power-Bolt+ has a low profile finished hex head and a full size thick bearing sleeve to provide increased capacity in shear connections.

GENERAL APPLICATIONS AND USES

• Structural connections, i.e., beam and column anchorage

Seismic Attachments (SDC A - F)

Cracked concrete / tension zone applications

- Conveyors and Material Handling
- Base Plates and Racking
- Guards, Bumpers and Barriers
- Mounting Machinery
- FEATURES AND BENEFITS
- + Consistent performance in high and low strength concrete
- + Anchor design allows for follow-up expansion after setting under tensile loading
- + Drill bit size is the same as the nominal anchor diameter
- + Anchor can be installed through standard fixture holes
- + Length ID code and identifying marking stamped on head of each anchor
- + High shear load capacity
- + Low profile finished hex bolt head
- + DEWALT dust removal drilling system (with HEPA dust extractor) can be used for an OSHA 1926.1153 Table 1 compliant solution

APPROVALS AND LISTINGS

- International Code Council, Evaluation Service (ICC-ES), ESR-3260 for cracked and uncracked concrete; code complaint with the 2021 IBC/IRC, 2018 IBC/IRC, 2015 IBC/IRC and 2012 IBC/IRC
- Tested in accordance with ACI 355.2/ASTM E488 and ICC-ES AC193 for use in structural concrete under the design provisions of ACI 318 (-19 or -14) Chapter 17 or ACI 318-11 Appendix D
- Evaluated and qualified by an accredited independent testing laboratory for recognition in cracked and uncracked concrete including seismic and wind loading (anchor category 1 for 1/2" to 3/4" sizes)
- City of Los Angeles, LABC Supplement (within ESR-3260)

GUIDE SPECIFICATIONS

CSI Divisions: 03 16 00 - Concrete Anchoring and 05 05 19 - Post-Installed Concrete Anchors Expansion anchors shall be Power-Bolt+ as supplied by DEWALT, Towson, MD. Anchors shall be installed in accordance with published instructions and the Authority Having Jurisdiction.

MATERIAL SPECIFICATIONS

Anchor Component	Carbon Steel Hex Head
Internal bolt	SAE Grade 8 equivalent $(f_y \ge 130,000 \text{ psi})$
Washer	Carbon steel, ASTM F844; meets dimensional requirements of ANSI B18.22.2, Type A Plain
Extension sleeve	Carbon Steel
Expansion clip	Carbon steel
Compression ring / Retention nut	Engineered plastic (Nylon)
Zinc plating	ASTM B633, SC1, Type III (Fe/Zn 5) – Mild service condition

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POWER-BOLT+ ASSEMBLY

HEAD STYLES

• Finished Hex Head

ANCHOR MATERIALS

 Zinc plated carbon steel bolt, washer, cone, sleeve, and expansion clip; assembled with a plastic compression ring and retainer nut

ANCHOR SIZE RANGE (TYP.)

• 1/4" through 3/4" diameters

SUITABLE BASE MATERIALS

- · Normal-weight concrete
- Lightweight concrete









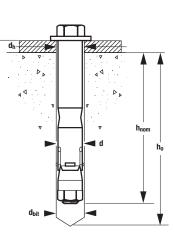
1-800-4 **DEWALT**



INSTALLATION SPECIFICATIONS

Power-Bolt+ Anchor Installation Specifications

Anchor Property/Setting	Notation	Units		Nominal Anchor Diameter (in.)						
Information	Notation	Units	1/4	3/8	1/2	5/8	3/4			
Anchor outside diameter	d	in. (mm)	0.250 (6.4)	0.375 (9.5)	0.500 (12.7)	0.625 (15.9)	0.750 (19.1)			
Internal Bolt Diameter (UNC)	-	in. (mm)	#8 (4)	1/4 (6.4)	3/8 (9.5)	7/16 (11.1)	9/16 (14.3)			
Nominal drill bit diameter (ANSI)	dbit	in.	1/4	3/8	1/2	5/8	3/4			
Minimum diameter of hole clearance in fixture	Сh	in. (mm)	5/16 (8)	7/16 (11)	9/16 (14)	11/16 (17)	13/16 (21)			
Minimum nominal embedment depth	hnom	in. (mm)	1-1/4 (32)	1-5/8 (41)	2-1/2 (64)	2-3/4 (70)	3 (76.2)			
Minimum hole depth	h₀	in. (mm)	h _{nom} + 1/4 (6)		h _{nom} + 3/8 (10)	h _{nom} -	+ 1/2 (13)			
Minimum member thickness	hmin	in. (mm)	3-1/2 (89)	4-1/2 (114)	5 (127)	6-1/2 (165)	7 (178)			
Minimum edge distance	Cmin	in. (mm)	1-3/4 (44)	2-3/4 (70)	3-1/4 (83)	4-1/2 (114)	6 (152)			
Minimum spacing distance	Smin	in. (mm)	2 (51)	3-1/2 (89)	4-1/2 (114)	6 (152)	5 (127)			
Installation torque	Tinst	ftlbf. (N-m)	4 (5)	20 (27)	40 (54)	60 (81)	110 (149)			
Torque wrench/socket size	-	in.	3/8	1/2	5/8	3/4	15/16			
Bolt Head Height	-	in. (mm)	1/8 (3)	13/64 (5)	9/32 (7)	5/16 (8)	3/8 (10)			
Washer O.D.	-	in.	7/16	47/64	1	1-1/4	1-15/32			



Head Marking



Legend

'PB+' Symbol = Power-Bolt+ Strength Design Compliant (see ordering information) Letter Code = Length Identification Mark

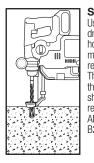


Length Identification

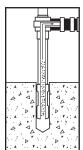
0																		
Mark	A	В	C	D	E	F	G	H	I	J	K	L	М	N	0	Р	Q	R
From	1-1/2"	2"	2-1/2"	3"	3-1/2"	4"	4-1/2"	5"	5-1/2"	6"	6-1/2"	7"	7-1/2"	8"	8-1/2"	9"	9-1/2"	10"
Up to but not including 2" 2-1/2" 3" 3-1/2" 4" 4-1/2" 5" 5-1/2" 6" 6-1/2" 7" 7-1/2" 8" 8-1/2" 9" 9-1/2" 10" 11"																		
Length identi	ength identification mark indicates the length of the anchor measured from under the washer to the end of the anchor.																	

INSTALLATION INSTRUCTIONS

Installation Instructions for Power-Bolt+ Anchor

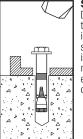


Step 1 Using the proper drill bit size, drill a hole into the base material to the required depth. The tolerances of the drill bit used should meet the requirements of ANSI Standard B212.15.

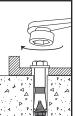


Step 2

Remove dust and debris from the hole during drilling (e.g. dust extractor, hollow bit) or following drilling (e.g. suction, forced air) to extract loose particles created by drilling. Ensure the cone is snug and uniformly under the expansion wedge (clip) with the clip fingers overlapping the anchor cone, prior to installation using the retention nut (see photo below).



Step 3 Drive anchor through the fixture into the hole. Be sure the anchor is driven to the minimum required embedment depth, hnom .



Step 4 Tighten the anchor with a torque wrench by applying the required installation torque, Tinst.



PERFORMANCE DATA (ASD)

Illtimate I oad	Canacities f	for Power-Bolt+ in	n Normal-Weight	Concrete ^{1,2}
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Nominal	Minimum		Minimum Concrete Compressive Strength												
Anchor Diameter	Embed. Depth	f'c = 2,500 p	si (17.3 MPa)	f'c = 3,000 p	si (20.7 MPa)	f'c = 4,000 p	si (27.6 MPa)	f'c = 6,000 p	si (41.4 MPa)	f'c = 8,000 psi (55.2 MPa)					
d in.	in. (mm) hnom	Tension Ibs. (kN)	Shear Ibs. (kN)	Tension Ibs. (kN)	Shear Ibs. (kN)	Tension Ibs. (kN)	Shear Ibs. (kN)	Tension Ibs. (kN)	Shear Ibs. (kN)	Tension Ibs. (kN)	Shear Ibs. (kN)				
	1-1/4	1,245	1,670	1,260	1,670	1,290	1,670	1,345	1,670	1,397	1,670				
1/4	(32)	(5.5)	(7.4)	(5.6)	(7.4)	(5.7)	(7.4)	(6.0)	(7.4)	(6.2)	(7.4)				
17 -1	1-3/4	1,740	1,670	1,905	1,670	1,945	1,670	1,945	1,670	1,945	1,670				
	(44)	(7.7)	(7.4)	(8.5)	(7.4)	(8.7)	(7.4)	(8.7)	(7.4)	(8.7)	(7.4)				
	1-5/8	1,420	2,420	1,555	2,460	1,795	2,460	2,105	2,470	2,430	2,810				
	(41)	(6.3)	(10.8)	(6.9)	(10.9)	(8.0)	(10.9)	(9.4)	(11.0)	(10.8)	(12.5)				
3/8	2	2,740	3,990	3,000	3,990	3,465	3,990	4,140	3,990	4,425	3,990				
0,0	(51)	(12.2)	(17.7)	(13.3)	(17.7)	(15.4)	(17.7)	(18.4)	(17.7)	(19.7)	(17.7)				
	2-3/4	4,130	3,990	4,425	3,990	4,425	3,990	4,425	3,990	4,425	3,990				
	(70)	(18.4)	(17.7)	(19.7)	(17.7)	(19.7)	(17.7)	(19.7)	(17.7)	(19.7)	(17.7)				
	2-1/2	3,880	7,420	4,250	8,030	4,905	8,030	5,150	8,030	5,518	8,030				
	(64)	(17.3)	(33.0)	(18.9)	(35.7)	(21.8)	(35.7)	(22.9)	(35.7)	(24.5)	(35.7)				
1/2	3	5,190	8,030	5,685	8,030	6,560	8,030	7,985	8,030	9,065	8,030				
	(76)	(23.1)	(35.7)	(25.3)	(35.7)	(29.2)	(35.7)	(35.5)	(35.7)	(40.3)	(35.7)				
	3-1/4	7,120	8,030	7,660	8,030	8,645	8,030	9,400	8,030	10,835	8,030				
	(83)	(31.7)	(35.7)	(34.1)	(35.7)	(38.5)	(35.7)	(41.8)	(35.7)	(48.2)	(35.7)				
	2-3/4	4,745	9,975	5,195	10,930	6,000	12,620	6,845	13,155	7,200	13,155				
	(70)	(21.1)	(44.4)	(23.1)	(48.6)	(26.7)	(56.1)	(30.4)	(58.5)	(32.0)	(58.5)				
5/8	3-1/2	6,995	9,975	7,660 (34,1)	10,930	8,845	12,620	11,325	13,155	12,900	13,155				
	(89)	(31.1)	(44.4)	1- /	(48.6)	(39.3)	(56.1)	(50.4)	(58.5)	(57.4)	(58.5)				
	3-3/4 (95)	8,710 (38.7)	12,015 (53.4)	9,545 (42.5)	14,320 (63.7)	11,020 (49.0)	16,535 (73.6)	12,820 (57.0)	18,250 (81.2)	14,800 (65.8)	18,250 (81.2)				
		· · · ·	· · · · ·		· · · ·		· · · /	· · · ·	· · · · ·	· · · · ·					
	3 (76)	5,655 (25,2)	10,950 (48,7)	6,195 (27.6)	11,995 (53.4)	7,155 (31.8)	13,850 (61.6)	8,385 (37.3)	18,510 (82.3)	9,685 (43.1)	21,370 (95.1)				
	4-3/8	(-)	(-)					· · · · ·		. ,	· · · /				
3/4	4-3/8 (111)	10,870 (48.4)	18,635 (82.9)	11,910 (53.0)	20,415 (90.8)	13,750 (61.2)	23,575 (104.9)	14,705 (65.4)	23,575 (104.9)	16,975 (75.5)	23,575 (104.9)				
	7	18.145	24,290	19,880	24,290	22,955	24,290	<u>`</u>	24,290	29.863	24,290				
	(178)	(80.7)	(108.0)	(88.4)	(108.0)	(102.1)	(108.0)	28,445 (126.5)	(108.0)	(132.8)	(108.0)				
	(170)	(00.7)	((00.4)	(100.0)	(102.1)	(100.0)	(120.0)	(100.0)	(132.0)	(100.0)				

1. The tabulated load values are applicable to single anchors installed in uncracked concrete with no edge or spacing considerations. Concrete compressive strength must be at the specified minimum at the time of installation.

2. Ultimate load capacities must reduced by a minimum safety factor of 4.0 or greater to determine allowable working loads.

Allowable Load Capacities for Power-Bolt+ in Normal-Weight Concrete^{1,2,3}

Nominal	Minimum	Minimum Concrete Compressive Strength												
Anchor Diameter	Embed. Depth	f'c = 2,500 p	si (17.3 MPa)	f'c = 3,000 p	si (20.7 MPa)	f'c = 4,000 p	si (27.6 MPa)	f'c = 6,000 p	si (41.4 MPa)	f ⁱ c = 8,000 p	si (55.2 MPa)			
d in.	in. (mm) hnom	Tension Ibs. (kN)	Shear Ibs. (kN)	Tension Ibs. (kN)	Shear Ibs. (kN)	Tension Ibs. (kN)	Shear Ibs. (kN)	Tension Ibs. (kN)	Shear Ibs. (kN)	Tension Ibs. (kN)	Shear Ibs. (kN)			
	1-1/4	310	420	315	420	325	420	335	420	350	420			
1/4	(32)	(1.4)	(1.9)	(1.4)	(1.9)	(1.4)	(1.9)	(1.5)	(1.9)	(1.6)	(1.9)			
1/4	1-3/4	435	420	475	420	485	420	485	420	485	420			
	(44)	(1.9)	(1.9)	(2.1)	(1.9)	(2.2)	(1.9)	(2.2)	(1.9)	(2.2)	(1.9)			
	1-5/8	355	605	390	615	450	615	525	620	610	705			
	(41)	(1.6)	(2.7)	(1.7)	(2.7)	(2.0)	(2.7)	(2.3)	(2.8)	(2.7)	(3.1)			
3/8	2	685	1,000	750	1,000	865	1,000	1,035	1,000	1,105	1,000			
0,0	(51)	(3.0)	(4.4)	(3.3)	(4.4)	(3.8)	(4.4)	(4.6)	(4.4)	(4.9)	(4.4)			
	2-3/4	1,035	1,000	1,105	1,000	1,105	1,000	1,105	1,000	1,105	1,000			
	(70)	(4.6)	(4.4)	(4.9)	(4.4)	(4.9)	(4.4)	(4.9)	(4.4)	(4.9)	(4.4)			
	2-1/2	970	1,855	1,065	2,010	1,225	2,010	1,290	2,010	1,380	2,010			
	(64)	(4.3)	(8.3)	(4.7)	(8.9)	(5.4)	(8.9)	(5.7)	(8.9)	(6.1)	(8.9)			
1/2	3	1,300	2,010	1,420	2,010	1,640	2,010	1,995	2,010	2,265	2,010			
=	(76)	(5.8)	(8.9)	(6.3)	(8.9)	(7.3)	(8.9)	(8.9)	(8.9)	(10.1)	(8.9)			
	3-1/4	1,780	2,010	1,915	2,010	2,160	2,010	2,350	2,010	2,710	2,010			
	(83)	(7.9)	(8.9)	(8.5)	(8.9)	(9.6)	(8.9)	(10.5)	(8.9)	(12.1)	(8.9)			
	2-3/4	1,185	2,495	1,300	2,735	1,500	3,155	1,710	3,290	1,800	3,290			
	(70)	(5.3)	(11.1)	(5.8)	(12.2)	(6.7)	(14.0)	(7.6)	(14.6)	(8.0)	(14.6)			
5/8	3-1/2 (89)	1,750 (7.8)	2,495 (11.1)	1,915	2,735	2,210	3,155	2,830 (12.6)	3,290 (14.6)	3,225 (14.3)	3,290 (14.6)			
	()	1 -7	· · /	(8.5)	(12.2)	(9.8)	(14.0)	· · · · ·	· · · ·	· · · · ·	· · · · ·			
	3-3/4 (95)	2,180 (9.7)	3,005 (13.4)	2,385 (10.6)	3,580 (15.9)	2,755 (12.3)	4,135 (18.4)	3,205 (14.3)	4,565 (20.3)	3,700 (16.5)	4,565 (20.3)			
	3	· · · ·	2.740				· · · /		· · · ·					
	(76)	1,415 (6.3)	(12.2)	1,550 (6.9)	3,000 (13.3)	1,790 (8.0)	3,465 (15.4)	2,095 (9.3)	4,630 (20.6)	2,420 (10.8)	5,345 (23.8)			
	4-3/8	2,720	4,660	2,980	5,105		5,895	3,675	5,895	4,245	5,895			
3/4	(111)	(12.1)	(20.7)	(13.3)	(22.7)	3,440 (15.3)	(26.2)	(16.3)	(26.2)	(18.9)	(26.2)			
	7	4,535	6,075	4,970	6,075	5.740	6,075	7,110	6,075	7,465	6,075			
	(178)	(20.2)	(27.0)	(22.1)	(27.0)	(25.5)	(27.0)	(31.6)	(27.0)	(33.2)	(27.0)			
1 Allowable I	/	sted are calculate		. /	· · · /	(20.0)	(27.0)	(01.0)	(27.0)	(00.2)	(27.0)			

1. Allowable load capacities listed are calculated using an applied safety factor of 4.0.

2. Tabulated load values are for anchors installed in uncracked concrete. Concrete compressive strength must be at the specified minimum at the time of installation.

3. Allowable load capacities are multiplied by reduction factors when anchor spacing or edge distances are less than critical distances.

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Load Adjustment Factors for Normal-Weight Concrete

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Spacing Reduction Factors - Tension (F _{NS})									
Diameter (in)		1/4	3/8	1/2	5/8	3/4			
Nominal Embedme	nt h _{nom} (in)	1-1/4	2	2-1/2	2-3/4	3			
Minimum Spacing	Smin (in)	2	3-1/2	4-1/2	6	5			
	2	0.78	-	-	-	-			
	2-1/2	0.82	-	-	-	-			
	3	0.87	-	-	-	-			
	3-1/2	0.91	0.80	-	-	-			
	4	0.96	0.83	-	-	-			
	4-1/2	1.00	0.86	0.83	-	-			
ches	5	1.00	0.89	0.85	-	0.77			
(inc	5-1/2	1.00	0.92	0.88	-	0.79			
ance	6	1.00	0.95	0.91	0.85	0.81			
Spacing Distance (inches)	6-1/2	1.00	0.98	0.93	0.87	0.83			
	7	1.00	1.00	0.96	0.90	0.85			
pac	7-1/2	1.00	1.00	0.98	0.92	0.87			
0	8	1.00	1.00	1.00	0.95	0.89			
	8-1/2	1.00	1.00	1.00	0.97	0.92			
	9	1.00	1.00	1.00	1.00	0.94			
	9-1/2	1.00	1.00	1.00	1.00	0.96			
	10	1.00	1.00	1.00	1.00	0.98			
	10-1/2	1.00	1.00	1.00	1.00	1.00			

Edge Distance	e Reducti	on Fac	tors - T	ension	(F _{NC})	
Diameter (in)		1/4	3/8	1/2	5/8	3/4
Nominal Embedmer	nt h _{nom} (in)	1-1/4	2	2-1/2	2-3/4	3
Minimum Edge Distance cmin (in)		1-3/4	2-3/4	3-1/4	4-1/2	6
	1-3/4	0.39	-	-	-	-
	2	0.44	-	-	-	-
	2-1/2	0.56	-	-	-	-
	3	0.67	0.46	-	-	-
•	3-1/4	0.72	0.50	0.41	-	-
Edge Distance (inches)	3-1/2	0.78	0.54	0.44	-	-
e (in	4	0.89	0.62	0.50	-	-
ance	4-1/2	1.00	0.69	0.56	0.75	-
Dist	5	1.00	0.77	0.63	0.83	-
dge	5-1/2	1.00	0.85	0.69	0.92	-
Ш	6	1.00	0.92	0.75	1.00	0.75
	6-1/2	1.00	1.00	0.81	1.00	0.81
	7	1.00	1.00	0.88	1.00	0.88
	7-1/2	1.00	1.00	0.94	1.00	0.94
	8	1.00	1.00	1.00	1.00	1.00

Spacing Reduction Factors - Shear (Fvs)

spacing neur	paciny neululi raciors - Silear (rvs)											
Diameter	(in)	1/4	3/8	1/2	5/8	3/4						
Nominal Embedm	ent hnom (in)	1-1/4	2	2-1/2	2-3/4	3						
Minimum Spaci	ng smin (in)	2	3-1/2	4-1/2	6	5						
	2	0.86	-	-	-	-						
	2-1/2	0.89	-	-	-	-						
	3	0.92	-	-	-	-						
	3-1/2	0.94	0.88	-	-	-						
	4	0.97	0.90	-	-	-						
	4-1/2	1.00	0.91	0.89	-	-						
Spacing Distance (inches)	5	1.00	0.93	0.91	-	0.84						
(inc	5-1/2	1.00	0.95	0.93	-	0.86						
Ince	6	1.00	0.97	0.94	0.89	0.87						
Dista	6-1/2	1.00	0.99	0.96	0.91	0.88						
l gui	7	1.00	1.00	0.97	0.93	0.90						
paci	7-1/2	1.00	1.00	0.99	0.94	0.91						
0	8	1.00	1.00	1.00	0.96	0.93						
	8-1/2	1.00	1.00	1.00	0.98	0.94						
	9	1.00	1.00	1.00	1.00	0.96						
	9-1/2	1.00	1.00	1.00	1.00	0.97						
	10	1.00	1.00	1.00	1.00	0.99						
	10-1/2	1.00	1.00	1.00	1.00	1.00						

Edge Distance Reduction Factors - Shear (Fvc)

Euge Distance Reduction Factors - Shear (FVC)										
Diameter (in)	1/4	3/8	1/2	5/8	3/4				
Nominal Embedme	ent hnom (in)	1-1/4	2	2-1/2	2-3/4	3				
Minimum Edge Dista	nce cmin (in)	1-3/4	2-3/4	3-1/4	4-1/2	6				
	1-3/4	0.39	-	-	-	-				
	2	0.44	-	-	-	-				
	2-1/2	0.56	-	-	-	-				
	3	0.67	0.44	-	-	-				
	3-1/4	0.72	0.48	0.41	-	-				
	3-1/2	0.78	0.52	0.44	-	-				
	4	0.89	0.59	0.51	-	-				
les)	4-1/2	1.00	0.67	0.57	0.50	-				
inch	5	1.00	0.74	0.63	0.56	-				
lce	5-1/2	1.00	0.81	0.70	0.61	-				
Edge Distance (inches)	6	1.00	0.89	0.76	0.67	0.57				
je Di	6-1/2	1.00	0.96	0.83	0.72	0.62				
Edi	7	1.00	1.00	0.89	0.78	0.67				
	7-1/2	1.00	1.00	0.95	0.83	0.71				
	8	1.00	1.00	1.00	0.89	0.76				
	8-1/2	1.00	1.00	1.00	0.94	0.81				
	9	1.00	1.00	1.00	1.00	0.86				
	9-1/2	1.00	1.00	1.00	1.00	0.90				
	10	1.00	1.00	1.00	1.00	0.95				
	10-1/2	1.00	1.00	1.00	1.00	1.00				

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Heavy Duty Sleeve Anchor

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ER-BO

STRENGTH DESIGN INFORMATION

Power-Bolt+ Anchor Installation Specifications in Concrete and Supplemental Information 1

Anchor Property/Setting Information		Notation	Units	Nominal Anchor Diameter (in.)						
				1/2	5/8	3/	4			
Anchor outside diameter	da	in. (mm)	0.500 (12.7)	0.625 (15.9)	0.7 (19					
Internal bolt diameter (UNC)		-	in. (mm)	3/8 (9.5)	7/16 (11.1)	9/16 (14.3)				
Minimum diameter of hole clearance in fixtur	e	dh	in. (mm)	9/16 (14.3)	11/16 (17.5)	13/16 (21.6)				
Nominal drill bit diamet	ter (ANSI)	d _{bit}	in.	1/2	5/8	3/4				
Minimum nominal embedment depth		hnom	in. (mm)	3-1/4 (83)	3-3/4 (95)	4-3/8 (111)				
Effective embedment		hef	in. (mm)	2-5/8 (67)	3 (76)	3-1/2 (89)				
Minimum hole depth		h _{hole}	in. (mm)	3-3/4 (95)	4-1/4 (108)	5 (127)				
Minimum member thickness		h _{min}	in. (mm)	5 (127)	6-1/2 (165)	7 (178)				
Minimum overall anchor length ²		lanch	in. (mm)	3-1/2 (89)	4 (102)	4-1/2 (114)				
Minimum edge distance		Cmin	in. (mm)	3-1/4 (83)	4-1/2 (114)	6 8 (152) (203)				
Minimum spacing distance		Smin	in. (mm)	4-1/2 (114)	6 (152)	6 5 (152) (127)				
Installation torque		Tinst	ftlbf. (N-m)	40 (54)	60 (81)	110 (149)				
Bolt Head Height		-	in. (mm)	9/32 (7.1)	5/16 (7.9)	3/8 (9.6)				
Torque wrench/socket size		-	in.	5/8	3/4	15/16				
Washer O.D.		-	in.	1	1-1/4	1-15/32				
Minimum specified yield strength		fy	psi (N/mm²)	130,000 (896)	130,000 (896)	130,000 (896)				
Minimum specified ultimate tensile strength [®]		f _{uta}	psi (N/mm²)	150,000 (1,034)	150,000 (1,034)	150,000 (1,034)				
Effective tensile stress area (internal bolt threads)		A _{se, N}	in² (mm²)	0.0775 (50)	0.1063 (68.6)	0.1820 (117.4)				
Effective shear stress area (internal bolt shank)		A _{se, v}	in² (mm²)	0.1069 (69)	0.1452 (93.7)	0.2410 (153.0)				
Moon avial atiffaces4	Uncracked concrete	$eta_{ ext{uncr}}$	lbf/in. (kN/mm)	366,000 (63)	871,000 (150)	256,000 (44)				
Mean axial stiffness⁴	Cracked concrete $eta_{ m cr}$		lbf/in. (kN/mm)	64,000 (11)	94,000 (16)	27,000 (5)				

For SI:1 inch = 25.4 mm, 1 ft-lbf = 1.356 N-m.

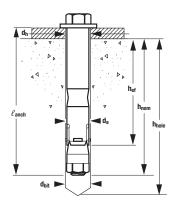
1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318 (-19 or -14) Chapter 17 or ACI 318-11 Appendix D.

2. The listed minimum overall anchor length is based on anchor sizes available at the time of publication compared with the requirements for the minimum nominal embedment depth and fixture attachment. The actual minimum anchor length must be determined by the taking the selected nominal embedment depth (e.g. required to obtain desired load capacity) and adding the thickness of the fixture, including any spacers or shims.

3. The maximum fixture thickness, tmax for selected anchors can be determined by taking the length of the selected anchor and subtracting the nominal embedment into the base material.

4. Mean values shown, actual stiffness varies considerably depending on concrete strength, loading and geometry of application.

Power-Bolt+ Anchor Detail



Tension Design information for Power-Bolt+ Anchor in Concrete



		Ï	Nominal Anchor Diameter				
Design Characteristic	Notation	Units	1/2 5/8 3/4				
Anchor category	1.2 or 3	-	1	1	1		
	,	in.	3-1/4	3-3/4	4-3/8		
Nominal embedment depth	h _{nom}	(mm)	(83)	(95)	(111)		
Effective embedment	h _{ef}	in. (mm)	2.625 (67)	3.00 (76)	3.50 (89)		
STEEL STRENGTH IN TENS	ON (ACI 318	-19 17.6.1, A	CI 318-14 17.4.1 or ACI 31	8-11 D.5.1)⁴			
Steel strength in tension	Nsa	lb (kN)	9,685 (43.1)	13,285 (59.1)	27,300 (121.4)		
Reduction factor for steel strength ³	φ	-	0.75 0.65				
CONCRETE BREAKOUT STRENGTH	IN TENSION (ACI 318-19 1	7.6.2, ACI 318-14 17.4.2 o	or ACI 318-11 D.5.2) ⁷			
Effectiveness factor for uncracked concrete	Kucr	-	27	27	24		
Effectiveness factor for cracked concrete	k _{cr}	-	17	17	17		
Modification factor for cracked and uncracked concrete ⁵	$\psi_{ ext{c,N}}$	-	1.0	1.0	1.0		
Critical edge distance (uncracked concrete only)	Cac	in. (mm)	8 (203)	6 (152)	8 (203)		
Reduction factor for concrete breakout strength ⁴	φ	-	0.65 (Condition B)				
PULLOUT STRENGTH IN TEN	ISION (ACI 31	8-19 17.6.3,	ACI 318-14 17.4.3 or ACI 3	318-11 D.5.3) ⁷			
Characteristic pullout strength, uncracked concrete (2,500 psi)	N _{p,uncr}	lb (kN)	Not Applicable6	Not Applicable6	Not Applicable6		
Characteristic pullout strength, cracked concrete (2,500 psi)	N _{p,cr}	lb (kN)	Not Applicable ⁶ Not Applicable ⁶		Not Applicable6		
Reduction factor for pullout strength	ϕ	-	0.65 (Condition B)				
PULLOUT STRENGTH IN TENSION FOR SEISM	IC APPLICATIO	ONS (ACI 318	-19 17.10.3, ACI 318-14 1	7.2.3.3 or ACI 318-11 D.3.3	3.3) ⁷		
Characteristic pullout strength, seismic (2,500 psi)	N _{p,eq}	lb (kN)	Not Applicable ⁶ Not Applicable ⁶ Not Applicable ⁶				
Reduction factor for pullout strength	φ	-	0.65 (Condition B)				

For SI: 1 inch = 25.4 mm; 1 ksi = 6.894 N/mm^2 ; 1 lbf = 0.0044 kN.

1. The data in this table is intended to be used with the design provisions of ACI 318 (-19 or -14) Chapter 17 or ACI 318-11 Appendix D, as applicable; for anchors resisting seismic load combinations the additional requirements of ACI 318-19 17.10, ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable, shall apply.

2. Installation must comply with the manufacturer's published installation instructions.

3. The tabulated value of ϕ for steel strength applies when the load combinations of Section 1605.2 of the IBC, ACI 318 (-19 or -14) Section 5.3 or ACI 318-11 Section 9.2, as applicable, are used. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ for steel strength must be determined in accordance with ACI 318-11 D.4.3. The anchors are ductile steel elements as defined in ACI 318 (-19 or -14) 2.3 or ACI 318-11 D.1, as applicable, except for the 3/4-inch-diameter, which is considered a brittle steel element for the purposes of design.

4. The tabulated value of \$\phi\$ for concrete breakout strength applies when both the load combinations of Section 1605.2 of the IBC, ACI 318 (-19 or -14) Section 5.3 or ACI 318-11 Section 9.2, as applicable, are used and the requirements of ACI 318-19 17.5.3, ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, for Condition B are satisfied. If the load combinations of Section 1605.2 of the IBC, ACI 318 (-19 or -14) Section 5.3 or ACI 318-19 17.5.3, ACI 318-11 D.4.3, as applicable, for Condition B are satisfied. If the load combinations of Section 9.2, as applicable, for Condition A are satisfied, the appropriate value of \$\phi\$ for concrete breakout strength must be determined in accordance with ACI 318-19 17.5.3, ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, for concrete breakout strength must be determined in accordance with ACI 318-19 17.5.3, ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable. If the load combinations of ACI 318-11 D.4.3, as applicable. If the load combinations of ACI 318-11 D.4.3, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of \$\phi\$ for concrete breakout strength must be determined in accordance with ACI 318-19 17.5.3, ACI 318-14 17.3.3 or ACI 318-11 D.4.4.

5. For all design cases use $\Psi_{cN} = 1.0$. The appropriate effectiveness factor for cracked concrete (ker) or uncracked concrete (kurar) must be used.

6. Pullout strength does not control design and does not need to be calculated for indicated size and embedment.

7. Anchors are permitted to be used in lightweight concrete provided the modification factor λ_n equal to 0.8 λ is applied to all values of $\sqrt{f'c'}$ affecting N_n and V_n. λ shall be determined in accordance with the corresponding version of ACI 318.

8. In accordance with ACI 318-19 17.6.1.2 and Eq. 17.6.1.1, ACI 318-14 17.4.1.2 and Eq. 17.4.1.2 or ACI 318-11 D.5.1.2 and Eq. D-2, as applicable, the nominal steel strength in tension is calculated using a limited value of fute of 125 ksi.

Shear Design information for Power-Bolt+ Anchor in Concrete



Design Characteristic	Notation	Units	Nominal Anchor Diameter					
	notation	Units	1/2	5/8	3/4			
Anchor category		-	1	1	1			
Nominal embedment depth		in. (mm)	3-1/4 3-3/4 (83) (95)		4-3/8 (111)			
Effective embedment	h _{ef}	in (mm)	2.625 3.000 (675) (76)		3.500 (89)			
STEEL STRENGTH IN SHEAR (ACI 318-19 17.7.1, ACI 318-14 17.5.1 or ACI 318-11 D.6.1)								
Steel strength in shear ^e	V _{sa}	lb (kN)	6,005 (26.7)	13,415 (59.7)	14,820 (65.9)			
Reduction factor for steel strength ³	ϕ	-	0.	0.60				
STEEL STRENGTH IN SHEAR FOR SEISMIC APPLICATIONS (ACI 318-19 17.10.1, ACI 318-14 17.2.3.3 or ACI 318-11 D.3.3.3)								
Steel strength in shear, seismic ⁸	V _{sa, eq}	lb (kN)	4,565 7,425 (20.3) (33.0)		14,820 (65.9)			
Reduction factor for steel strength in shear for seismic ³	ϕ	-	0.65 0.60					
CONCRETE BREAKOUT STRENGTH IN SHEAR (ACI 318-19 17.7.2, ACI 318-14 17.5.2 or ACI 318-11 D.6.2) ⁷								
Load bearing length of anchor	le	in (mm)	1.00 1.25 (25) (32)		1.50 (38)			
Nominal anchor diameter	da	in (mm)	0.500 0.625 (12.7) (15.9)		0.750 (19.1)			
Reduction factor for concrete breakout ⁴	φ	-	0.70 (Condition B)					
PRYOUT STRENGTH IN SHEAR (ACI 318-19 17.7.3, ACI 318-14 17.2.3.3 or ACI 318-11 D.6.3) ⁷								
Coefficient for pryout strength	Kcp	-	2.0 2.0		2.0			
Reduction factor for pryout strength ⁵	φ	-	0.70 (Condition B)					
For SI: 1 inch = 25.4 mm; 1 ksi = 6.894 N/mm ² ; 1 lbf = 0.0044 kN.	•		*					

For SI: 1 inch = 25.4 mm; 1 ksi = 6.894 N/mm²; 1 lbf = 0.0044 kN.

1. The data in this table is intended to be used with the design provisions of ACI 318 (-19 or -14) Chapter 17 or ACI 318-11 Appendix D, as applicable; for anchors resisting seismic load combinations the additional requirements of ACI 318-19 17.10, ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable, shall apply.

2. Installation must comply with the manufacturer's published installation instructions.

3. The tabulated value of ϕ for steel strength applies when the load combinations of Section 1605.2 of the IBC, ACI 318 (-19 or -14) Section 5.3 or ACI 318-11 Section 9.2, as applicable, are used. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ for steel strength must be determined in accordance with ACI 318-11 D.4.3. The anchors are ductile steel elements as defined inACI 318 (-19 or -14) 2.3 or ACI 318-11 D.1, as applicable, except for the 3/4-inch-diameter which is considered a brittle steel element for the purposes of design.

4. The tabulated value of φ for concrete breakout strength applies when both the load combinations of Section 1605.2 of the IBC, ACI 318 (-19 or -14) Section 5.3 or ACI 318-11 Section 9.2, as applicable, are used and the requirements of ACI 318-19 17.5.3, ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, for Condition B are satisfied. If the load combinations of Section 1605.2 of the IBC, ACI 318 (-19 or -14) Section 5.3 or ACI 318-19 17.5.3, ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, for Condition B are satisfied. If the load combinations of Section 1605.2 of the IBC, ACI 318 (-19 or -14) Section 5.3 or ACI 318-11 Section 9.2, as applicable, are used and the requirements of ACI 318-19 Section 17.5.3, ACI 318-11 Section 9.2, as applicable, are used and the requirements of ACI 318-19 Section 17.5.3, ACI 318-11 D.4.3, as applicable, for Condition A are satisfied, the appropriate value of φ for concrete breakout strength must be determined in accordance with ACI 318-11 D.4.3, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of φ for concrete breakout strength must be determined in accordance with ACI 318-11 D.4.4.

5. The tabulated value of for pryout strength applies if the load combinations of Section 1605.2 of the IBC, ACI 318 (-19 or -14) Section 5.3 or ACI 318-11 Section 9.2, as applicable, are used. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of *d* for pryout strength must be determined in accordance with ACI 318-11 D.4.4, for condition B.

6. Tabulated values for steel strength in shear must be used for design. The tabulated values for the shear stress area are listed conservatively and the results for the steel strength will be more conservative when using ACI 318-19 Section 17.7.1.2 and Eq. 17.7.1.2a, ACI 318-14 Eq. 17.5.1.2b or ACI 318-11 Eq. D-29, as applicable.

7. Anchors are permitted to be used in lightweight concrete provided the modification factor λ_a equal to 0.8 λ is applied to all values of \sqrt{fc} affecting N_n and V_n. λ shall be determined in accordance with the corresponding version of ACI 318.

8. Tabulated values for steel strength in shear are for seismic applications and based on test results in accordance with ACI 355.2, Section 9.6.

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DESIGN STRENGTH TABLES (SD)

Tension and Shear Design Strengths for Power-Bolt+ in Cracked Concrete^{1,2,3,4,5,6,7,8}



Concrete Breakout Strength Controls

Tension and Shear Design Strengths for Power-Bolt+ in Uncracked Concrete^{1,2,3,4,5,6,7}

		Minimum Concrete Compressive Strength, f'c (psi)										
Nominal Anchor	Nominal Embed.	f'c = 2,	f'c = 2,500 psi		f'c = 3,000 psi		f'c = 4,000 psi		f'c = 6,000 psi		f'c = 8,000 psi	
Diameter (in.)	h (in.)	ϕ Nn Tension (lbs.)	ØVn Shear (Ibs.)	ϕ Nn Tension (lbs.)	ØVn Shear (lbs.)	ØNn Tension (lbs.)	ØVn Shear (Ibs.)	ϕ Nn Tension (lbs.)	ØVn Shear (Ibs.)	ϕ Nn Tension (lbs.)	∲Vn Shear (lbs.)	
1/2	3-1/4	3,730	3,905	4,090	3,905	4,720	3,905	5,780	3,905	6,675	3,905	
5/8	3-3/4	4,560	7,145	4,995	7,830	5,770	8,720	7,065	8,720	8,155	8,720	
3/4	4-3/8	5,105	8,890	5,595	8,890	6,460	8,890	7,910	8,890	9,135	8,890	
- Concrete Breakout Strength Controls - Steel Strength Controls												

Tabular values are provided for illustration and are applicable for single anchors installed in normal-weight concrete with minimum slab thickness, ha = 1.5*hmin, and 1with the following conditions:

- c_{a1} is greater than or equal to 1.5 times the critical edge distance, c_{ac} (table values based on $c_{a1} = 1.5^{+}c_{ac}$).

Ca2 is greater than or equal to 1.5 times Ca1.

Calculations were performed according to ACI 318 (-19 or -14) Chapter 17. The load level corresponding to the controlling failure mode is listed. (e.g. For tension: 2steel, concrete breakout and pullout; For shear: steel, concrete breakout and pryout). Furthermore, the capacities for concrete breakout strength in tension and pryout strength in shear are calculated using the effective embedment values, her, for the selected anchors as noted in the design information tables. Please also reference the installation specifications for more information.

Strength reduction factors (ø) were based on ACI 318 (-19 or -14) Section 5.3 for load combinations. Condition B is assumed. 3-

Tabular values are permitted for static loads only, seismic loading is not considered with these tables. 4-

For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318 (-19 or -14) Chapter 17. 5-

Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths please see ACI 318 (-19 or -14) Chapter 17. 6-For other design conditions including seismic considerations please see ACI 318 (-19 or -14) Chapter 17.

The tabulated design strengths may be converted to allowable stress design values. Divide by conversion factor calculated as a weighted average of the load factors 7for the controlling load combination.

8-For seismic design in accordance with ACI 318, the tabulated tension design strengths in cracked concrete for concrete breakout and pullout must be multiplied by a factor of 0.75.

Cat. No.

6902SD-PWR

6906SD-PWR

6911SD-PWR

6910SD-PWR

6913SD-PWR

6914SD-PWR

6916SD-PWR

6930SD-PWR

6932SD-PWR

6934SD-PWR

6936SD-PWR

6940SD-PWR

6942SD-PWR

6944SD-PWR

6945SD-PWR

6947SD-PWR

6950SD-PWR

6952SD-PWR

6954SD-PWR

6956SD-PWR

ORDERING INFORMATION

Power-Bolt+ (Carbon Steel with Finished Hex Head)

Anchor Size

1/4" X 1-3/4"

1/4" X 3"

3/8" x 1-7/8"

3/8" X 2-1/4"

3/8" X 3"

3/8" X 3-1/2"

3/8" X 4"

1/2" x 2-3/4"

1/2" x 3-1/2"

1/2" x 4-3/4"

1/2" x 5-3/4"

5/8" x 3"

5/8" x 4"

5/8" x 5"

5/8" x 6"

5/8" x 8-1/2"

3/4" x 3-1/4"

3/4" x 4-1/2"

3/4" x 5-1/4"

3/4" x 7-1/4"

Approximate Maximum

Fixture Thickness

1/2"

1-3/4"

1/4"

1/4"

1"

1-1/2"

2"

1/4"

1/4"

1-1/2"

2-1/2"

1/4"

1/4"

1-1/4"

2-1/4"

4-3/4"

1/4"

1/8"

7/8"

2-7/8"

Pack

Qty.

100

100

50

50

50

50

50

50

50

25

25

20

15

15

15

10

15

10

10

10

Carton

Qty.

600

600

300

300

300

300

300

200

200

150

150

120

90

90

90

40

90

60

60

40

Full Head SDS-Plus

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DW5527

DW5527

DW5527

DW5527

DW5527

DW5537

DW5537

DW5537

DW5537

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SDS-Plus

DW5427

DW5427

DW5427

DW5427

DW5427

DW5429

DW5429

DW5429

DW5429

DW5446

DW5446

DW5446

DW5446

DW5447

DW5453

DW5453

DW5453

DW5453

CHANICAL ANCHORS

POWER-BOLT®+ Heavy Duty Sleeve Anchor

 6957SD-PWR
 3/4" x 8-1/4"
 3-7/8"
 10
 40
 DW5455
 DW5809
 DWA54034
 DWA54034

 Shaded catalog numbers denote sizes which are too small or lengths less than the minimum standard anchor length for strength design. Anchors not long enough to meet the minimum nominal embedments published for strength design are outside the scope of ICC-ES ESR-3260.
 DW5455
 DW5809
 DWA54034
 DWA54034

Suggested ANSI Carbide Drill Bit Cat, No.

SDS-Max

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_

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DW5803

DW5803

DW5803

DW5803

DW5806

DW5806

DW5806

DW5806

DW5809

DW5809

DW5809

DW5809

DW5809

Hollow Bit SDS-Plus

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DWA54012

DWA54012

DWA54012

DWA54012

DWA54058

DWA54058

DWA54058

DWA54058

DWA54058

DWA54034

DWA54034

DWA54034

DWA54034

Hollow Bit SDS-Max

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DWA54058

DWA54058

DWA58058

DWA58058

DWA58058

DWA54034

DWA54034

DWA54034

DWA54034

The published size includes the diameter and the length. The length is measured from below the washer to the end of the anchor.

The tabulated maximum fixture thickness is provided for reference and based on published nominal embedment depths. The actual maximum fixture thickness for the anchor is determined by subtracting the required nominal embedment depth for the application from the published length.

To determine the actual minimum anchor length, select the nominal embedment depth needed (e.g. required to obtain desired load capacity). Then add the thickness of the fixture, including any spacers or shims, to the embedment depth.

Hollow drill bits must be used with a dust extraction vacuum (e.g. Cat. No. DW012).

1-800-4	DEWALT
1-000-4	

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